

(Collapse Analysis of Frames using Finite Element Limit Method)

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Key Words : Collapse analysis (), Load-carrying capacity (), Frame structure (), Finite element limit method (),

ABSTRACT : This paper discusses the collapse behavior of frames under quasi-static loading conditions. Based on the duality theorem in plasticity, the finite element limit analysis for three-dimensional structures is formulated. The analysis considers sequential deformation of frames with work-hardening effects. The collapse analysis of frames is conducted using finite element limit analysis program, and its results are compared with experiments. With various step sizes, the example studies demonstrate that the results show good agreement with each other in load-carrying capacity.

q : method) (semi-empirical)
 σ_{ij} :
 σ_0 : 가
 $\bar{\sigma}$:
 ε_{ij} :
 $\bar{\varepsilon}^p$: ,
 t_i :
 u_i : (step size)
 $\| \cdot \|_{(v)}$: von-Mises norm
 $\| \cdot \|_{(-v)}$: Negative von-Mises norm
 K : (analytical solution)
 U :
 C : 가 (2),
 A : (penalty function value)
 λ :
 n : (3,4)
 1. , 3 가 , 가
 가 (1)
 가

$$\int_D \sigma_{ij} \varepsilon_{ij} d\Omega = q \int_{\partial D_S} t_i u_i d\Gamma \quad (3)$$

2.

2.1

$$q = \frac{\int_D \sigma_{ij} \varepsilon_{ij} d\Omega}{\int_{\partial D_S} t_i u_i d\Gamma} \quad (4)$$

(4) , u 가 (normalization)

가

가

가

가

(5)

$$\int_{\partial D_S} t_i u_i d\Gamma = 1 \quad (5)$$

Hölder

, $\sigma_{ij} \varepsilon_{ij}$

$$\sigma_{ij} \varepsilon_{ij} = |\sigma_{ij} \varepsilon_{ij}| \leq \|\sigma\|_{(v)} \|\varepsilon\|_{(-v)} = \bar{\sigma} \bar{\varepsilon} \quad (6)$$

maximize $q(\sigma)$

subject to $\nabla \cdot \sigma = 0$ in D

$\sigma_{ji} n_j = q t_i$ in ∂D_S

$\|\sigma\|_{(v)} \leq \sigma_0$ in D

(1)

$q(\sigma)$

가

$$\begin{aligned} q(\sigma) &= \int_D \sigma_{ij} \varepsilon_{ij} d\Omega \\ &\leq \int_D \|\sigma_{ij}\|_{(v)} \|\varepsilon_{ij}\|_{(-v)} d\Omega \\ &\leq \sigma_0 \int_D \|\varepsilon_{ij}\|_{(-v)} d\Omega \\ &= q(\tilde{u}) \end{aligned} \quad (7)$$

가

minimize $q(\tilde{u})$

subject to $q = \sigma_0 \int_D \|\varepsilon_{ij}\|_{(-v)} d\Omega$

$$\int_{\partial D_S} t_i u_i d\Gamma = 1 \quad (8)$$

$$Tr(\varepsilon_{ij}) = 0$$

Kinematic boundary conditions

(8) $\tilde{q}(u)$

$$\int_D u_i \sigma_{ji,j} d\Omega = 0, \forall u_i \quad (2)$$

(1) $q(\sigma)$

(2) 2.2

$$q(\tilde{u}) = \bar{\sigma} \int_D \|\varepsilon_{ij}\|_{(-v)} d\Omega + A \int_D u_{i,i}^2 d\Omega \quad (9)$$

, A
(9)

$$\bar{\varepsilon} = \sqrt{\frac{2}{3} \varepsilon_{ij} \varepsilon_{ij}} = \sqrt{[U]^T [K_1^e] [U]} \quad (10)$$

$$\varepsilon_v = \varepsilon_x + \varepsilon_y + \varepsilon_z = [K_2^e] [U] \quad (11)$$

(10) (11) (9)

$\tilde{q}(u)$

$$\tilde{q}(u) = \sum_{e=1}^E \{ [U]^T [\hat{K}_1^e] [U] + [U]^T [\hat{K}_2^e] [U] \} \quad (12)$$

$$[\hat{K}_1^e] = \bar{\sigma} \int_{D_e} [K_1^e] d\Omega \quad (13)$$

$$[\hat{K}_2^e] = A \int_{D_e} [K_2^e]^T [K_2^e] d\Omega \quad (14)$$

(12)

$$([\hat{K}_1^e])_n = \int_D \frac{([K_1^e])_n}{\left(\sqrt{[U]^T [K_1^e] [U]} \right)_{n-1}} d\Omega \quad (15)$$

, n-1 , n
(8)

$$\begin{aligned} & \text{minimize } \tilde{q}(U) = [U]^T [K] [U] \\ & \text{subject to } [C]^T [U] = 1 \end{aligned} \quad (16)$$

2.3

(16)

$$\begin{aligned} & \text{minimize } \Phi(U) = [U]^T [K] [U] \\ & \quad - 2\lambda ([C]^T [U] - 1) \end{aligned} \quad (17)$$

n

$$[U]_n = \lambda [K]^{-1} [C] = \frac{[K]^{-1} [C]}{[C]^T [K]^{-1} [C]} \quad (18)$$

$$\tilde{q}_n = [U]^T [K] [U] = \lambda_n \quad (19)$$

2.4 가

가

가

(bisection method)

(6.7.8)

가

가

$$\bar{\sigma} = \sigma_o (1 + A \bar{\varepsilon}^p)^n \quad (20)$$

3.

340MPa,
350MPa . Fig. 1 2

Fig. 3 Fig.

4

(tip)

1mm

가

0.2% 2%, 10%

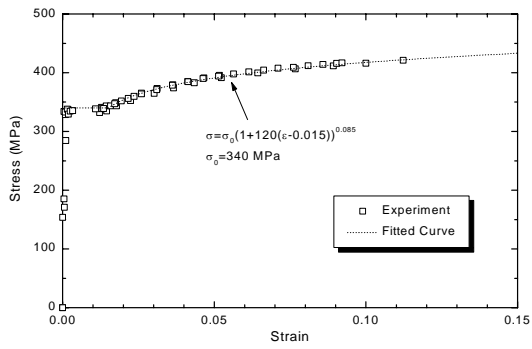


Fig. 1 Effective stress–strain relation for a square frame.

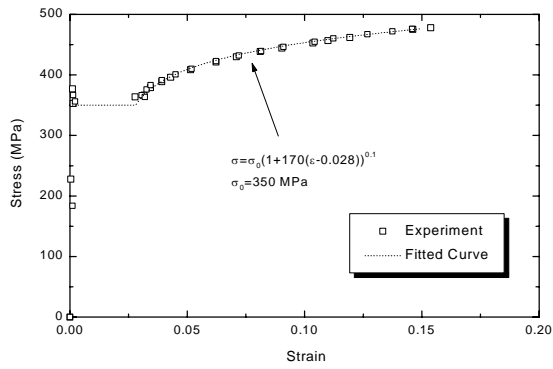


Fig. 2 Effective stress–strain relation for an arc frame.

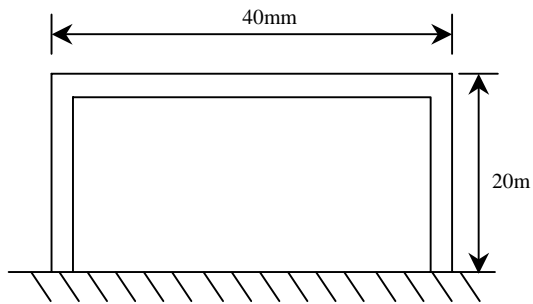


Fig. 3 Geometry of an arc frame.

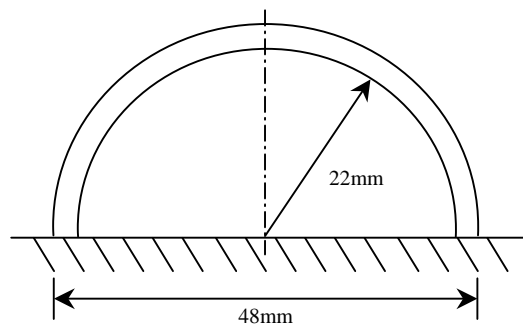


Fig. 4 Geometry of a square frame.

3.1

1.35mm
Fig. 5

1.35mm ×

가

2% 10%

가

0.2%,
가

가 가

Fig. 6

가 10mm

3.2

2mm

가

Fig. 7

2mm ×

Fig. 8

가 10mm

4.

0.2%, 2% 10%

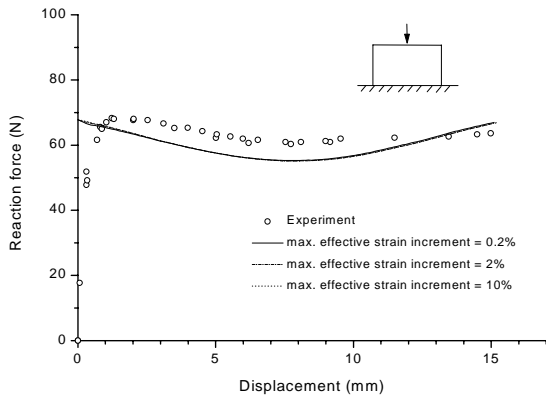
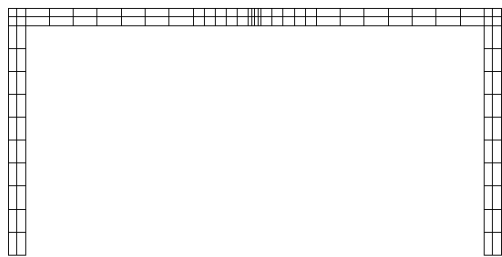
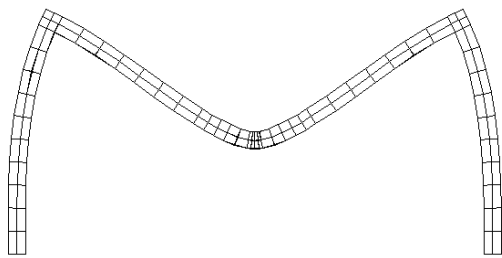


Fig. 5 Collapse load of a square frame.



(a) undeformed shape



(b) deformed shape when displacement = 10mm

Fig. 6 Undeformed and deformed shape of a square frame.

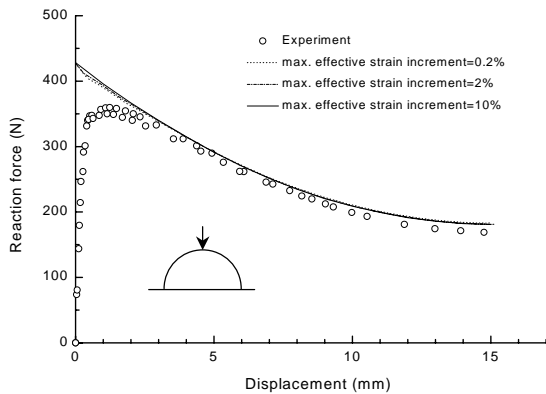
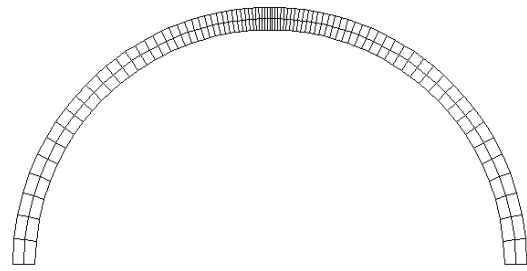
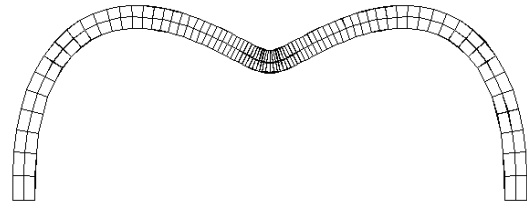


Fig. 7 Collapse load of an arc frame.



(a) undeformed shape



(b) deformed shape when displacement = 10mm

Fig. 8 Undeformed and deformed shape of an arc frame.

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